Field Evaluation of Virus Treatment Efficiency by Wastewater Soil Absorption Systems with Aggregate-Free and Aggregate-Laden Infiltrative Surfaces

Final Report

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ACRONYMS AND ABBREVIATIONS

A _{I.S.}	-	area of the infiltrative surface
Bkgd	-	background
BOD ₅	-	biochemical oxygen demand at 5-days
cBOD ₅	-	carbonaceous biochemical oxygen demand at 5-days
CSM	-	Colorado School of Mines
cfu	-	colony forming unit
CV	-	coefficient of variation
D	-	depth
FC	-	fecal coliforms
HLR	-	hydraulic loading rate
ID	-	inner diameter
Id#	-	identification number
IS	-	infiltrative surface
ISI	-	Infiltrator Systems, Inc.
LTAR	-	long term acceptance rate
MPI	-	minutes per inch
MS-2	-	bacteriophage that infects E. coli.
ND	-	non detect
Ne	-	effective porosity
PBS	-	phosphate buffered saline solution
PETG	-	poly ethylenterephthalate
pfu	-	plaque forming unit
PRD-1	-	bacteriophage that infects Salmonella typhimurium
SCS	-	Soil Conservation Service
STE	-	septic tank effluent
Q	-	flow rate
T _r	-	travel time required
USDA	-	U.S. Department of Agriculture
WSAS	-	wastewater soil absorption system(s)

ACKNOWLEDGMENTS

A research program to characterize the hydraulic and purification performance of mature onsite wastewater systems (i.e., systems that have been in operation for more than one year), including systems with aggregate-free (chamber) and aggregate-laden (gravel) infiltrative surfaces is underway in the Environmental Science and Engineering Division at the Colorado School of Mines. A major element of this research program was completed in the Fall of 1999 which included Phase 1 field sampling and analysis of onsite wastewater treatment systems at 16 homes in Colorado and ancillary laboratory studies (Siegrist et al., 2000). This report describes the methods and results of a companion field study, Phase 2, completed in Colorado to evaluate the field performance of four wastewater soil absorption systems including both aggregate-free (chamber) and aggregate-laden (gravel) designs. This work was made possible by the contributions and support of several individuals and organizations. The work was sponsored by Infiltrator Systems, Inc. and ISI is acknowledged for their interest and support in advancing the state-of-knowledge regarding onsite wastewater system design and performance. This study would not have been possible without the participation and cooperation of the homeowners at the individual residences studied. In addition, the following individuals are recognized for their contributions and valuable support:

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1.0 EXECUTIVE SUMMARY

This report discusses a field study completed at the Colorado School of Mines (CSM) to characterize the hydraulic and purification performance of mature onsite wastewater systems (i.e., systems that have been in operation for more than one year), including systems with aggregate-free (chamber) and aggregate-laden (gravel) infiltrative surfaces. This work included Phase 1 field testing of 16 mature soil infiltration systems including one system for virus treatment efficiency in 1999 and Phase 2 field testing for system characterization and virus treatment efficiency at four mature soil infiltration systems in 2001. Phase 2 field testing was conducted at four individual home sites, from the pool of 16 homes monitored during Phase 1, located in Adams and Summit Counties, Colorado. The four sites included one aggregate-laden wastewater soil absorption system (WSAS) and three aggregate-free chamber WSAS. Results of the Phase 1 field study have been previously reported (Siegrist et al. 2000, Siegrist and Van Cuyk 2001).

At four of the study sites, septic tank effluent (STE) was characterized, and three intact soil cores, two from within the infiltrative surface and one background location, were successfully acquired and the soil properties were characterized with depth. Effluent and soil core samples show constituent levels in the range of previously reported work. As expected, levels of nitrogen species and fecal coliform bacteria decreased with increased depth below the infiltrative surface.

To evaluate virus treatment efficiency, a multicomponent mixture of viral surrogates (two bacteriophages, MS-2 and PRD-1) and a conservative tracer (bromide) were added to the STE before it was applied to the soil absorption system. Samples of the STE were collected every 2 to 3 days and characterized for the surrogate and tracer concentrations. Seven to nine days after introduction of the viral surrogates and tracer, coring of the subsurface beneath the infiltrative surface commenced. Soil cores were taken at three spatially separate locations and subsamples were collected at depths of 0-5, 10-15, 25-30 and 55-60 cm below the infiltrative surface. Extraction and analysis were conducted for bromide, MS-2, PRD-1, fecal coliform, nutrients (total nitrogen and total phosphorous), and water content.

The methodology for using a multicomponent mixture of virus surrogates and a conservative tracer to assess virus purification in a wastewater soil absorption system developed during Phase 1 was successfully applied under field conditions at four additional sites in Phase 2. Bromide was detected in all but one soil core from within the infiltrative surface. Three-log reductions in the applied MS-2 and PRD-1 viral surrogate concentrations were achieved at 30 cm below the infiltrative surface.

The results of this study suggest that aggregate-free systems in Colorado that are sized with 40 to 50% less gross infiltration area for a given design flow are providing equivalent environmental protection as the larger aggregate-laden systems. These field results are consistent with the results of intermediate-scale 3-D sand lysimeter studies (Van Cuyk et al. 1999, Siegrist et al. 1999, Van Cuyk et al. 2001) and Phase 1 field testing (Siegrist et al. 2000, Siegrist and Van Cuyk 2001) performed previously at CSM.

2.0 INTRODUCTION

2.1. BACKGROUND

Wastewater treatment for onsite and small community applications commonly relies on infiltration and percolation of primary effluent through soil to achieve purification prior to recharge to ground water (U.S. EPA 1978, 1980, 1997, Jenssen and Siegrist 1990, Crites and Tchobanoglous 1998) (Fig. 2.1). These wastewater soil absorption systems (WSAS) can achieve high purification efficiencies due to the complex interactions of hydraulic and purification processes (Fig. 2.2) (Schwagger and Boller 1997, Ausland 1998, McCray et al. 2000). Extensive and lengthy contact between wastewater constituents and the soil matrix and associated biofilms occurs during unsaturated flow achieved by daily loadings limited to a small fraction of the soil's saturated hydraulic conductivity (K_{sat}) (e.g., <5 cm/d). In addition, a clogging zone evolves at the soil infiltrative surface (Fig. 2.1 and 2.2) which leads to reduced permeability and more uniform infiltration and a concomitant unsaturated flow almost regardless of hydraulic loading. Wastewater-induced clogging increases the soil biogeochemical activity and can enhance sorption, biotransformation and die-off/inactivation processes within the clogging zone itself or in the underlying unsaturated soil (Siegrist 1987, Siegrist et al. 1991, Ausland 1998, Van Cuyk et al. 1999, McCray et al. 2000). Clogging zone genesis has been described as a humification-like process and modeled as a function of the mass loading rates of wastewater suspended matter and bio-oxidizable substances (Siegrist 1987, Siegrist and Boyle 1987). In most WSAS, clogging zone genesis must occur to some degree to foster the advanced purification required before ground water recharge, but not to the point where it causes hydraulic problems.



Fig. 2.1. Illustration of an onsite wastewater soil absorption system typical of the 25 million systems in operation in the U.S. today.

Fig. 2.2. Illustration of hydraulic and purification processes operative in a wastewater soil absorption system.

If clogging zone development is retarded or absent altogether, for example due to the application of highly pretreated effluent (e.g., sand filter effluent), purification of pathogens and other constituents of concern may be less than predicted and desired. Conversely, if soil clogging is too excessive, for example due to application of high strength effluents (e.g., restaurant wastewater), clogging can be detrimental by causing hydraulic dysfunction and soil anaerobiosis and reduced purification (e.g., slower organic matter breakdown and reduced nitrification).

System physical features, operational parameters, and environmental conditions can determine hydraulic and purification behaviors in wastewater infiltration systems. As briefly described below, the infiltrative surface character and the underlying unsaturated soil depth above a ground water table (i.e., vadose zone), are two system features that are commonly determined during design. The soil infiltrative surface is normally located below the original ground surface and commonly has a 15- to 30-cm thick layer of 2- to 4-cm diameter gravel placed on it to provide storage for peak wastewater flows and to support the overburden soils (Fig. 2.1). Performance data regarding the rate and extent of soil clogging in systems with gravel on the infiltrative surface (aggregate-laden) led to system designs that avoid the use of gravel aggregate (e.g., open chamber, fabric-wrapped piping, plastic media, fabric bundles). The most common type of system that provides an open or aggregate-free surface involves the use of chambers (Keys 1996, May 1996, Tyler et al. 1991).

Gravel on an infiltrative surface can reduce infiltration zone permeability (or infiltrability) by (1) blocking pore entries, (2) becoming embedded in the soil matrix, (3) yielding fines that are deposited in pore entries, or (4) focusing wastewater constituents as a result of the reduced permeability due to the effects of (1)-(3) (Amerson et al., 1991, Caldwell Connell Engineers Pty 1986, Jenssen and Siegrist 1990, Siegrist 1987, Siegrist and Boyle 1987, Siegrist et al. 1991, Tyler and Converse 1994). Based on an equivalency concept with respect to infiltrability, aggregate-free systems are being utilized with design infiltration areas (i.e., gross total area provided) on the order of 40 to 50% less than required with aggregate-laden systems. While keeping the daily loading rate onto the effective infiltrative area the same (i.e., that surface not masked or impacted directly by gravel), the relative hydraulic loading rate (HLR) on the gross infiltration area is increased by 67 to 100%. While previous experience with aggregate-free systems has revealed satisfactory hydraulic performance (Dix and May 1997, England and Dix 1999, Hoxie et al. 1990, May 1996, Nodarse & Assoc., Inc. 1997, Ricklefs 1992, Saxton and Crumley 1993), until recently, comparatively less experimental data has existed regarding purification performance (Van Cuyk et al. 1999).

The depth of the soil vadose zone to ground water can affect hydraulic function and in turn purification by influencing the soil water content, aeration status, media surface area, and hydraulic retention time. In the U.S., depths for soil infiltration systems range from 0.15 (South Carolina) to 1.5 m (Massachusetts) and for intermittent sand filters, from 0.6 and 0.9 m (US EPA 1980, Anderson et al. 1985, Crites and Tchobanoglous 1998, ISI 2001). While a high degree of treatment normally occurs in the infiltration zone as soil clogging develops, at higher HLRs and with nonuniform distribution methods, constituents of concern that would normally be treated may be transported through the vadose zone to ground water. For example, many studies have shown that a large percentage of bacteria remain near the infiltrative surface when effluents are applied to porous media (Brown et al. 1979, Kristiansen 1991, Huysman and Verstraete 1993,

Emerick et al. 1997, Masson 1999, Stevik et al. 1999). However, if HLRs are too high or the dosing frequency is too low, some microbes can be transported to lower regions in a soil matrix, posing a purification concern in systems that are too shallow to ground water. Alternatively, while depth is important to hydraulic and purification behavior, at some point there is limited gain in purification by increasing vadose zone depth (Peeples et al. 1991).

In Colorado, wastewater soil absorption systems (WSAS) are designed based on long-term acceptance rates (LTAR) and a design flow estimated at 225 gpd/bedroom (based on 75 gpcd * 2 per/bedroom* 150% peaking factor). The LTAR ranges are typical of those reported in the literature and used in other state codes (e.g., coarse to medium sand w/ 1-5 minute per inch (MPI) percolation test = 1.3 gpd/ft², fine to loamy sand w/ 6-10 MPI = 1.2 gpd/ft², sandy loam to loam w/ 11-20 MPI = 0.72 gpd/ft², loam w/21-30 MPI = 0.50 gpd/ft², silt loam to sandy clay loam w/ 31-40 MPI = 0.40 gpd/ft², sandy clay loam w/ 41-60 MPI = 0.3 gpd/ft², etc.). Thus, design application rates in Colorado are mostly in the range of 0.40 to 1.2 gpd/ft². The required distance between the bottom of an infiltration trench or bed and high ground water is 4 ft. Regulations allow infiltration area to include sidewall area (below the distribution pipe) and bottom area. The State allows a 50% reduction in area with a "standard" Infiltrator chamber (12 in. high, 34 in. wide and with a 7 in invert height) compared to an aggregate-laden trench. When used in a bed configuration, the State of Colorado allows for a 40% area reduction using this "standard" Infiltrator chamber. In each case, the reduction is based on the actual open area of the chamber bottom.

2.2. GOALS, OBJECTIVES AND SCOPE

Research was initiated in the Environmental Science & Engineering Division at the CSM to study the hydraulic and purification behavior of wastewater soil infiltration systems from startup through initial clogging zone development and to quantify the effects of infiltrative surface character and vadose zone soil depth. The research effort is comprised of four primary efforts; (1) controlled laboratory experimentation with 3-dimensional lysimeters, (2) Phase 1 field testing of 16 mature soil infiltration systems including 1 system for virus treatment efficiency, (3) Phase 2 field testing of virus treatment efficiency at 4 mature soil infiltration systems, and (4) transport/fate and process modeling.

The methods and results of the 3-D lysimeter studies are described in previous publications (e.g., Fischer 1999, Masson 1999, Van Cuyk et. al. 1999, Siegrist and Van Cuyk 2001, Van Cuyk et al. 2001). Results of these 3-D laboratory lysimeter studies completed in 1999 revealed that the performance of aggregate-free systems was comparable to aggregate-laden systems, even though the HLR of the aggregate-free system is 67% higher (i.e., 8.4 vs. 5.0 cm/day, respectively, based on gross horizontal area provided). For both system types, it was shown that a 60- to 90-cm depth to groundwater provided adequate depth of unsaturated media for purification of conventional pollutants (e.g., cBOD₅, suspended solids, ammonia nitrogen) as well as bacteria and virus to occur.

Two phases of field studies have been conducted. Phase 1, initiated in 1999 was designed to complement the lysimeter studies by focusing on infiltration of domestic STE in more mature

soil absorption systems under field conditions (Siegrist et al. 2000, Siegrist and Van Cuyk 2001). In this study WSAS in Colorado that had been in operation for one year or longer were characterized through STE and soil sampling and analyses to gain insight into the comparative performance of aggregate-free (chamber) and aggregate-laden (gravel) infiltration systems. A total of 16 individual onsite wastewater systems were monitored including 10 chamber systems and 6 gravel systems. Data collected at each site included residence characteristics, system design features, STE composition, occurrence and depth of ponding of the soil infiltrative surface, and pollutant concentrations with depth below the infiltrative surface for parameters such as nutrients and fecal coliform bacteria. During Phase 1, a laboratory study was completed to determine the relationship between fecal coliform bacteria concentrations measured directly in percolating water versus analyses of bulk soil samples. Finally, virus treatment efficiency was evaluated using a multicomponent mixture of MS-2 and PRD-1 bacteriophages and a conservative bromide tracer. The results of these Phase 1 studies suggest that aggregate-free systems in Colorado that are sized with 40 to 50% less infiltration area for a given design flow are performing comparably to the larger aggregate-laden systems. These field results are consistent with the results of intermediate-scale sand lysimeter studies performed previously at CSM.

Phase 2 field testing, described in this report, was initiated in Fall 2000 as a companion to Phase 1 to expand the evaluation of virus treatment efficiency from one chamber WSAS to a total of 5 WSAS. The purpose of this Phase 2 research is to evaluate and characterize the hydraulic and purification performance of mature onsite wastewater systems (i.e., systems that have been in operation for more than one year), including systems with aggregate-free (chamber) and aggregate-laden (gravel) infiltrative surfaces. As in Phase 1, virus treatment efficiency was evaluated using a multicomponent field tracer test including a conservative tracer (bromide) and two viral surrogates (MS-2 and PRD-1). Field testing was conducted at 4 individual home sites located in Brighton and Summit County in Colorado from the pool of 16 homes monitored during Phase 1. The four sites included one aggregate-laden WSAS and three aggregate-free (chamber) WSAS. Both water and soil samples were retrieved from the field with analysis completed in the CSM pilot laboratory and at an off-site laboratory (University of Wisconsin Soil and Plant Analysis Laboratory).

This report describes the methods and results of the Phase 2 field work. Section 3 contains a description of the technical approach and methods while Section 4 summarizes the virus treatment study results. Section 5 presents the conclusions and recommendations derived from the work.

3.0 TECHNICAL APPROACH AND METHODS FOR VIRUS TREATMENT EFFICIENCY

3.1. HOME IDENTIFICATION AND CHARACTERISTICS

Study subdivisions in Colorado were identified during Phase 1 based on individual expressions of interest to collaborate with the CSM research team by county environmental health department staff as well as subdivision developers and homeowner's associations. The two study areas identified included the Hamilton Creek subdivision in Summit County, Colorado and the Todd Creek Farms subdivision near Brighton in Adams County, Colorado. The Hamilton Creek subdivision is located approximately 60 miles west of Denver, near the town of Silverthorne and is at an elevation of ~9,000 feet. Todd Creek Farms subdivision, located 40 miles northeast of Denver, is at an elevation of ~5,000 feet.

Initially working with the environmental health departments in Summit County and Tri-County, Colorado, individual homes within each of the two study subdivisions were identified by letters of invitation to participate and by support from key subdivision persons (e.g., president of homeowners association). Homeowner questionnaires were used to gather information regarding (1) dwelling occupancy and water using and waste generating fixtures and (2) general soil absorption system design features. General screening criteria were established so the study would include individual wastewater systems that were: designed and installed according to modern practice; between 1 and 10 years old; and loaded at ≥ 25 to 50% of the design flow capacity. Detailed information regarding the onsite system site evaluation, design, and installation was gathered from county records, as-built construction drawings and interviews with homeowners, as well as field observations. From this, a pool of 16 systems was identified that had been in operation for periods of 1 to 5 years or longer and included both aggregate-free Infiltrator chamber systems and aggregate-laden gravel systems (Siegrist et al. 2000 Siegrist and Van Cuyk 2001).

In order to build off of the information obtained during Phase 1 field testing, and to ensure that the systems evaluated in both studies were comparable, a letter of invitation to participate in Phase 2 was mailed to each of the 16 Phase 1 participants. From these 16 systems, a total of 7 responses (3 aggregate-laden and 4 aggregate-free) to the invitation were received including 6 interested in participating and 1 no longer interested in participating (homeowners had moved). Final selection for participation in Phase 2 was based on maturity of the onsite system, exhibited STE ponding of the infiltrative surface, and sites providing easy access to the septic tank and chamber soil absorption system to facilitate the required sampling activities. Based on these selection criteria and to ensure evaluation of both aggregate-laden and aggregate-free Infiltrator chamber systems, 1 aggregate-laden gravel system from Hamilton Creek and 3 aggregate-free Infiltrator chamber systems from the Todd Creek Farms subdivision were selected for study during Phase 2. It should be noted that of the three aggregate-laden systems interested in participating, one system was ponded above the gravel layer and one system could not be located by the field team due to rocky, frozen soil covering the system. "New" systems (i.e., systems other than those from the initial pool of 16 homes) were not pursued at this time in order to build off of the previous information gained (i.e., characterization and monitoring completed during Phase 1). At the 4 homes selected for inclusion in Phase 2, an initial site inspection was conducted, 4 STE samples were collected, multicomponent surrogate/tracer mixture was added to the system, and at least 3 soil cores (2 locations from within the infiltrative surface and one background location) were collected 7 to 9 days following surrogate/tracer addition.

All of the onsite systems included septic tank pretreatment with a dosed (but not uniform, pressure distribution) application of STE to WSAS trenches or narrow beds. Design practice in both Summit County and Adams County allows for 40 to 50% reduction in infiltrative surface of aggregate-free systems compared to aggregate-laden system, however; sizing of individual systems relies on subsurface soil infiltration rates. Some characteristics of the homes and their onsite systems are summarized in Table 3.1. General schematics of each home site are presented in Figs. A.1 through A.4.

Table 3.1.	Selected c	haracteristics	of onsite	e wastewater	systems	monitored i	n this study.
					-		2

Home site ¹	Res.	Water	Date of	I.S.	Effective	Est.	Est.	Ponded
Id#	size	use	system	type	Area of I.S.	HLR ²	HLR ²	
	(BR)	(gal/mon)	installa-	51	(ft^2)	(gpd/ft^2)	(cm/day)	
			tion				· · · ·	
2 (P1)	3	7785	1991	Chamber	354	0.73	2.99	Yes
10 (P2)	4	5350	1992	Gravel	864	0.21	0.84	Yes
12* (P2)	3	7550	1997	Chamber	918	0.28	1.16	Yes
14* (P2)	4	5092 ³	1997	Chamber	1140	0.15	0.61	No (wet)
16* (P2)	3	9880	1998	Chamber	656	0.53	2.16	Yes

P1 = Phase 1, completed in 1999.

P2 = Phase 2, completed in 2001.

¹ Homes identified and evaluated during Phase 1. * denotes homes located in Adams County.

² HLR = flow (gpd)/area of effective infiltrative surface (ft²) where 1 cm/d = 0.24 gpd/ft².

³ Range based on differing values reported on the as-built drawings and permit.

3.2. EVALUATION OF WSAS UNDER FIELD CONDITIONS

During Phase 1, one system, from the pool of 16 WSAS monitored, was chosen for field-scale evaluation of virus treatment efficiency using a multicomponent surrogate and tracer methodology. This effort was viewed as experimental and as a means to refine methodology employed during controlled laboratory experiments (Van Cuyk et al. 2001) and apply it under field conditions to a mature operating WSAS. Conservative tracers and viral surrogates had been used previously in studying flow and transport in ground water systems (both native bacteriophage and spiked phage) (Harvey 1997a, 1997b) and appeared quite suitable for evaluation of WSAS under field conditions. However, multicomponent surrogates and tracers had received relatively limited use for evaluation of WSAS under field conditions. Field studies reported in the literature included studies in Florida with virus surrogate spiking of a research site near Tampa by Anderson et al. (1991, 1994) and spiking of cesspools near the Florida Keys by Rose et al. (1999). Field studies completed in California by Oakely et al. (1999) and in Massachusetts by Higgins et al. (1999) relied on indigenous bacteriophage. Most of these studies (all but the study by Anderson et al. (1994)) did not employ multicomponent mixtures containing a conservative tracer plus two contrasting viral surrogates. During Phase 1, the methodology utilizing a multicomponent mixture of virus surrogates and a conservative tracer was successfully applied under field conditions with the results reported elsewhere (Siegrist et al. 2000, Siegrist and Van Cuyk 2001 Van Cuyk et al. 2001).

In this Phase 2 study, methods similar to those used during Phase 1 were employed to confirm that four additional mature WSAS, including both Infiltrator chambers with a typical reduced infiltrative surface area and aggregate-laden systems, can remove virus such that the concentrations are reduced by ≥ 3 logs between the infiltrative surface and 60 cm depth below the infiltrative surface. Two viral surrogates (MS-2 and PRD-1 bacteriophage) and a conservative tracer were added to the STE being applied to a soil absorption system at four sites. Then after 7 to 9 days, during which application of STE and the surrogates/tracer continued, soil cores were collected from the infiltrative surface vertically downward to 60 cm depth below the infiltrative surface. From each core, soil samples were aseptically collected and analyzed to quantify the concentrations of the viral surrogates and tracer as well as the soil water content, organic matter, nutrients, and fecal coliform concentrations.

3.2.1 Soil Characteristics

General soil characteristics for the two subdivision locations were initially assessed from the U. S. Department of Agriculture (USDA) Soil Conservation Survey (SCS) reports (USDA 1974, 1977). Soils in the Hamilton Creek subdivision near Silverthorne were reported to consist of Anvik and Frisco soils (mixed Boralfic and mixed Typic Cryoborolls) of deep, well drained material formed in colluvium and glacial drift derived from a variety of rocks (USDA 1977). These soils are on mountainous uplands that have slopes of 6-35%. The typical soil profile includes a brown loam surface layer (0-15 in., 0-38 cm) with a subsoil of clay loam (15-20 in., 38-51 cm). The depth to bedrock is >5 ft. (1.5 m) and depth to high ground water is > 6 ft. (1.8 m). Rock fragments (10-24 in. (25-60 cm) diameter) make up 30-80% of the solum. Soils in the Todd Creek Farms subdivision near Brighton consist of nearly level to strongly sloping, well drained Platner and Ulm loams (fine, montmorillonitic mesic soils) (USDA 1974). The typical soil profile includes a heavy loam surface layer (0-7 in., 0-18 cm), with a subsoil of silty clay (7-13 in., 18-33 cm) and a substratum of clay (13-22 in., 33-56 cm). It is listed by SCS as having slow permeability and an average depth to bedrock of 40 to 60 in. (1-1.5 m).

Soil characteristics specific to this study were determined from soil samples collected from the pool of 16 WSAS during Phase 1 at depths below the infiltrative surface. Analysis performed at CSM for grain size distribution revealed a coarse-grained soil texture which included considerable pebble and cobble fractions. As shown in Fig. 3.1, the grain size analyses revealed 35 wt.% gravel, 55 wt.% sand, and <10 wt.% of the soil in the silt and clay fraction (i.e., wt.% passing through a no. 200 sieve which is 0.074 mm diameter).



Fig. 3.1. Grain size distributions for soil samples collected from the infiltrative surface depths at study sites in two subdivisions monitored in Colorado.

3.2.2 Wastewater Flow and Hydraulic Loading Rate

Water use data were collected during Phase 1 via water use records and/or periodic readings of water meters at each home. The water use data were assumed to be representative of wastewater flow. The flow data were used along with the WSAS area determined from the as-built plans to calculate the estimated HLR in gpd/ft² that each WSAS was actually receiving (see Table 3.1). Water use data was not obtained at one location (home site Id# 14) as a community well serves several homes in the area and data specific to the home of interest was not available.

3.2.3 Septic Tank Effluent Sampling and Analysis

Septic tank effluent sampling and analysis during Phase 1 has been previously reported (Siegrist et al. 2000). During Phase 2, STE was collected from within the last compartment of the septic tank (all septic tanks were either 2- or 3-chamber tanks). Duplicate grab samples were taken and placed in sterile polypropylene bottles and stored at 4°C until brought to the laboratory for analysis (Fig. 3.2). Two rounds of STE samples were collected from each site prior to surrogate/tracer addition. All laboratory analysis of the STE were performed within 24 hours of sample collection. The following characteristics of the STE were determined following standard methods (APHA 1998).

- o pH was measured electrometrically.
- o Alkalinity was measured (total alkalinity) via titration with sulfuric acid according to APHA method 2320B.
- o cBOD₅ (carbonaceous biochemical oxygen demand) was measured according to APHA method 5210B.

- o COD analysis was performed using a Hach reactor digestion, colorimetric method (Hach 1992, U.S. EPA-approved).
- o Total solids and total suspended solids were measured according to APHA methods 2540B and 2540D.
- o Total nitrogen (TN) was measured by persulfate digestion, nitrate nitrogen by chromotropic acid method and ammonium by salicylate method (Hach 1992, U.S. EPA-approved).
- o Total phosphorus (TP) was measured according to EPA acid persulfate method (U.S. EPA 365.2).
- o Fecal coliform analysis was performed by membrane filtration according to APHA method 9222D.





3.2.4 Multicomponent Surrogate/Tracer Addition

The multicomponent mixture was comprised of two viral surrogates, MS-2 and PRD-1 bacteriophages (not infectious to humans) (Van Duin 1988), in addition to the conservative tracer, bromide. MS-2 and PRD-1 had been previously used as viral surrogates in ground water transport studies (Harvey 1997a,b) as well as Phase 1 (Siegrist et al. 2000, Van Cuyk et al. 2001). MS-2 is an icosahedral phage with a diameter of 26 nm (VanDuin 1988) and a pH_{iep} of 3.9 (Bales et al. 1991) while PRD-1 is an icosahedral lipid phage with a diameter of 62 nm (Olsen et al. 1974).

The volume of wastewater within the last compartment of the septic tank at each study site was estimated based on as-built drawings and other homeowner records. Prior to the addition of the surrogate/tracer mixture, STE samples, in triplicate, were collected to quantify pretreatment levels of bromide, MS-2 and PRD-1. All samples were analyzed in duplicate. Surrogates or tracer were not detected in the STE with the exception of one sample at 1 pfu-MS-2/mL. Stock solutions of bromide (added as KBr), MS-2 and PRD-1 were added to the last compartment in the septic tank to obtain final concentrations targeted at 1500 mg-Br/L of bromide and 1.0×10^7 pfu/mL of both MS-2 and PRD-1. The bacteriophage concentrations were selected to be

representative of those in STE during or soon after a viral infection within the household and enable detection in the soil.

Following surrogate/tracer addition to the STE, the tank was mixed using a submersible pump which recirculated STE within the compartment for approximately 10 minutes. After this period of mixing, five (5) grab samples of the STE, amended with the surrogates and tracer, were collected to characterize the time zero conditions. Subsequently, STE samples were collected from the dosing tank every 2 to 3 days in order to characterize the concentrations of surrogates and tracers being dosed into the soil infiltration system over time.

An estimate of the time required for effluent to infiltrate and percolate to a depth of 60 cm was made based on the daily flow, area of infiltrative surface, and an effective porosity for the soil based on the following relationship:

$$T_r = \frac{(A_{rs.})(D)(Ne)}{Q}$$
[3.1]

where, T_r = travel time required for effluent to reach the depth of interest (days), D = depth of interest (m), A_{IS} = infiltrative surface area (m²), Q = daily flow (m³/day), and Ne = effective porosity (v/v). This relationship assumes uniform application and infiltration into the absorption system and is thus a first approximation of travel times. For the Phase 2 study sites, the ALS was determined from the as-built drawings (see Table 3.1), the depth of interest to evaluate was 0.60 m, the average daily flow was $0.9 \text{ m}^3/\text{d}$, and the effective porosity was estimated at 0.20 v/v. For these conditions, the time required for applied effluent to percolate to 60 cm depth below the infiltrative surface was estimated at 5.5 to 20 days. However, Phase 1 results indicated that these travel times were conservative as indicated by the presence of detectable bacteriophage in the soil but not bromide. It was assumed that the bromide had either passed through the system during the time between tracer addition to the tank and coring (26 days) or to have been applied at too low of a concentration (i.e., 500 mg-Br/L) to be detected within the soil pore water. Building off of these Phase 1 observations, a higher concentration of bromide (1500 mg-Br/L) was initially added to the tank and coring commenced ~ 1 week following surrogate/tracer addition. During this time between surrogate/tracer addition and soil coring, samples were collected from the STE being applied to the WSAS to identify changes in concentrations over time.

3.2.5 WSAS Soil Coring and Sampling

Soil coring, sampling and analysis of the WSAS at 15 of 16 sites was completed during Phase 1 and has been reported previously (Siegrist et al. 2000). Soil coring, sampling and analysis was not completed at one site due to ponding above the gravel surface at the time of the field investigation. This site was not included in Phase 2.

During Phase 2, the WSAS were probed at two spatially separate locations within the infiltration systems and at one background location using dual-tube sampling methods 7 to 9 days after introduction of the surrogate/tracer (Fig. 3.3). At three locations (home site Id#s 10, 12, and 14)

the GeoProbeTM 54DT, a small gas powered direct-push probing machine, was used to collect soil cores (Fig. 3.4). At the forth location (home site Id# 16) soil cores were collected using hand methods similar to those used during Phase 1. For the chamber systems, the soil above the chamber was hand excavated and an access hole was cut into the top of the chamber for access to the infiltrative surface (Fig. 3.5). For the gravel system, a continuous soil core was collected from the ground surface to 60 cm below the infiltrative surface using the GeoProbeTM 54DT. Schematics of core locations at each site are presented in Appendix A (Fig. A-1 through A-4).





Fig. 3.3. Illustration of the general field site monitoring location.

Direct-push soil core collection with the GeoProbeTM 54DT utilized the GeoProbe/Terraprobe sampling method (Fig 3.6). A 4-ft.-long x 2-in.-inner diameter (ID) (1.2 m x 5 cm) dual-tube assembly with polyethylenterephthalate (PETG) liners was used to collect continuous

undisturbed samples from the ground surface to 24 in. (60 cm) below the infiltrative surface. The dual-tube assembly is comprised of an outer stainless steel core barrel (3.25-in ID [8 cm]), an inner stainless steel core barrel (2.25-in ID [6 cm]) and PETG liners (2-in ID [5 cm]) inserted into the inner core barrel. The dual-tube assembly is hammered, without rotation, \sim 3.5 ft (1 m) into the ground surface. The inner core barrel with PETG sleeve and soil core is then retrieved to the surface. Upon retrieval to the surface, the PETG liner with the intact soil core was removed from the sampler, capped and stored at 4°C prior to transporting to the CSM laboratory for analyses. A clean PETG sleeve was then replaced into the inner core barrel and reinserted into the outer core barrel retained in the subsurface. The dual-tube assembly was then again advanced \sim 3.5 ft (1 m) and the process repeated until a continuous core to the desired depth was obtained.

Hand sample collection methods consisted of hand excavating to the top of the chamber, cutting a hole in the top of the chamber and inserting a 4-in. (10 cm) ID PVC plastic pipe to case the hand excavated hole and prevent ponded fluid from entering the corehole. Care was taken upon encountering the soil infiltrative surface to avoid unnecessary disruption of the surface soil and any clogging layer. The ponded fluid was sampled and pumped with a peristaltic pump out of the center of the casing. Then, a thin-tube (1.75-in. ID [4.5 cm]) sampling probe with a precleaned acetate liner (6-in. long [15 cm]) was driven into the undisturbed soil and an intact core retrieved within the sleeve. The core was capped with foil, labeled, and placed in a cooler containing blue ice. The thin-tube push probe (after cleaning with 70% v/v ethanol in water followed by a deionized water rinse) was inserted into the probe hole and driven another depth interval (nominally 6 in. [15 cm]). This process was repeated until a depth of 24 in. (60 cm) was reached or cobbles and dense soil prevented further penetration.

These soil core collection methods enabled relatively intact core samples to be aseptically collected from the WSAS infiltrative surface vertically downward to a depth of 60 cm below the infiltrative surface. The occurrence and magnitude of ponding within each system was manually determined. All samples were stored at 4°C until laboratory analysis was performed at CSM.



Fig. 3.4 The GeoProbeTM 54DT direct-push mobile rig.



Fig. 3.5 Hand excavation from ground

surface to top of a chamber system.



Fig. 3.6. Soil core collection using direct-push methods: a) locating the equipment at the site, b) advancing the dual-tube assembly, and c) retrieval of dual-tube assembly with sample.

3.2.6 WSAS Soil Analysis

Following soil core collection in the field, samples were transported to the CSM laboratory for analysis. In the laboratory, the cores were carefully opened and the outer-most soil media was removed and wasted (Fig. 3.7). Subsamples of the interior of the core were then taken with sterile utensils at 4 intervals (0-5 cm, 10-15 cm, 25-30 cm, and 55-60 cm below the infiltrative surface) that corresponded approximately to those used in the previous CSM laboratory lysimeter study and Phase 1 sampling (Van Cuyk et al. 1999, Siegrist et al. 2000). At each sample interval, soil sample extractions were performed for bromide, bacteriophage, and fecal coliform bacteria within 24 hours of sample collection. Laboratory analyses for water content was also performed within 24 hours of sample collection.

shipped to the University of Wisconsin Soil and Plant Analysis Laboratory for organic matter and nutrients analyses.

Analyses of field core subsamples were made for the following characteristics (all soil results are expressed per gram or kilogram of dry soil):

- o Soil color was recorded using the Munsell Color Chart.
- o Water content was measured gravimetrically and recorded as percent dry weight.
- o Dried soil samples were also analyzed for pH, organic matter, total nitrogen, ammonium, nitrate, available phosphorus. Results were expressed on a dry weight basis.
- o Bromide was measured on a 1:1 (soil:DI) extract using ion chromatography.
- Fecal coliform analysis on soil core samples was performed aseptically by taking a known weight (~5 grams) of moist soil and adding 40 mL of 1.5% beef extract solution to a yield a final dilution of ~1:8 (soil:beef extract). APHA method 9221A suggests extraction for coliform bacteria in sediments and sludges using 10% phosphate buffered saline (PBS). However, a comparison of extraction methods conducted at the bench scale at the CSM microbiology laboratory using 6 different extractants (including PBS) proved beef extract to be the most efficient method for removing the coliform bacteria (Masson, 1999). Following the addition of beef extract, samples were shaken for 30 minutes at ~60 rpm and then allowed to settle for at least 15 minutes prior to analysis. An aliquot of liquid (3-10 mL) was withdrawn from mid-depth of a sterile 50 ml conical (Masson, 1999) and analyzed directly (for low levels) or diluted as needed (for high levels). Analyses for fecal coliform bacteria were made according to the membrane filtration method (APHA method 9222D).
- Bacteriophage, both MS-2 and PRD-1, analysis on soil core samples was performed on the extract (~1:8 soil:beef extract) prepared for fecal coliform analysis described above. MS-2 and PRD-1 bacteriophage assays were made following the plaque-forming-unit technique (*Escherichia coli* and *Salmonella typhimurium* host, respectively) described by Adams (1959).



Fig. 3.7 Soil sample preparation for analyses.

4. VIRUS TREATMENT EFFICIENCY RESULTS

4.1. WSAS CHARACTERISTICS

4.1.1 WSAS Age and HLR

A summary of the system characteristics is shown in Table 4.1. The chamber systems varied in age from 3 to 10 yr. while the gravel system was 9 yr. old. The estimated HLR averaged 1.73 cm/d for the chamber systems, including both Phase 1 and Phase 2, compared to 0.84 cm/d for the gravel system. During Phase 2, three of the four systems (home site Id#s 10, 12, and 16) exhibited some degree of effluent ponding while the fourth system (home site Id# 14) was wet with ponding observable in surface depressions. During soil coring (after surrogate/tracer addition) it was observed that one system (home site Id#16) was fully ponded. This system was not eliminated from consideration at this time due in part to schedule and reporting constraints (e.g., time required to identify an additional system, add surrogate/tracer to the STE, and allow sufficient time for migration of the surrogate/tracer into an through the infiltration area). It is also important to note that this system was designed as an infiltration/evapotransporation system, relying on a fraction of net loss of fluid from evapotransporation rather than percolation through the low permeable subsurface soils, and was sampled during the winter months when evapotransporation was at a minimum (i.e., dominant infiltration/negligible evapotransporation). Data from these four systems suggest comparable hydraulic performance with the chamber systems receiving a loading rate on average that was 106% higher (1.73 vs. 0.84 cm/d per unit of horizontal infiltrative surface area).

Site Id#	Phase of Sampling	Infiltrativ e Surface type	Itrativ Age Est. HLR Depth urface ype (cm/d) Surface		Depth to Infiltrative Surface	Ponding Depth
2	Phase 1	Chamber	10	2.99	~4 ft	yes ¹
10	Phase 2	Gravel	9	0.84	3.0 to 3.5 ft	~3 in.
12	Phase 2	Chamber	4	1.16	2.5 to 3.3 ft	2 in.
14	Phase 2	Chamber	4	0.61	1.3 to 1.6 ft	wet
16	Phase 2	Chamber	3	2.16	1.5 to 2.25 ft	12 in.

Table 4.1.Summary of system characteristics.

¹ Depth of ponding not measured.

4.1.2 Septic Tank Effluent Composition

Descriptive statistics for STE at each site are presented in Table 4.2 while results for individual homes may be found in the Appendix (see Tables A.1 through A.4). The STE composition at the individual study homes was typical of residential STE containing appreciable concentrations of pollutants. The average concentrations were: $cBOD_5 = 122 \text{ mg/L}$, TSS = 209 mg/L, total N = 58 mg-N/L, and total P = 13.4 mg-P/L. Fecal coliform bacteria ranged from 1.4 x 10⁵ to 2.64 x 10⁷ cfu/100mL. Table 4.3 presents a synopsis of some literature values, allowing a comparison of the STE composition determined in this study to previously reported values.

Parameter	Units	Average	Std. dev.	Total No.	Minimum	Maximum
рН	-	7.57	0.53	16	6.98	8.55
Alkalinity	mg-CaCO ₃ /L	535	42	16	464	620
cBOD ₅	mg/L	122	120	10	31	389
COD	mg/L	558	130	20	369	786
TS	mg/L	1191	157	32	630	1435
TSS	mg/L	209	127	31	35	550
TN	mg-N/L	58	16	32	32	84
NH ₄ -N	mg-N/L	47	13	32	29	75
NO ₃ -N	mg-N/L	0.66	0.18	31	0.4	1.1
Total P	mg-P/L	13.4	6.4	31	2.2	31.2
Fecal coli.	cfu/100mL	NA	NA	23	1.40E+05	2.64E+07

Table 4.2. Descriptive statistics for STE composition.

¹ See Appendix A, Tables A.1 through A.4 for detailed results of each WSAS.

NA = statistic is not applicable.

Parameter (units)	Average	Std. dev.	Range	Reference
BOD ₅	122	120	30 to 389	This CSM study, Phase 2
(mg/L)	175	52	98 to 358	This CSM study, Phase 1
	81	31	29 to 140	Tyler et al., 1991
	-	-	150 to 250	Crites & Tchobanoglous, 1998
	132	-	-	Harkin et al., 1979
COD	558	130	369 to 786	This CSM study, Phase 2
(mg/L)	260	164	109 to 990	This CSM study, Phase 1
	157	46	49 to 244	Tyler et al., 1991
	-	-	250 to 500	Crites & Tchobanoglous, 1998
	445	-	-	Harkin et al., 1979
TSS	209	127	35 to 550	This CSM study, Phase 2
(mg/L)	251	245	20 to 958	This CSM study, Phase 1
	-	-	40 to 140	Crites & Tchobanoglous, 1998
	87	-	-	Harkin et al., 1979
NH ₄ -N	47	13	29 to 75	This CSM study, Phase 2
(mg-N/L)	43	17	3 to 64	This CSM study, Phase 1
	50	11	10 to 69	Tyler et al., 1991
	41	-	30 to 50	Crites & Tchobanoglous, 1998
NO ₃ -N	0.66	0.18	0.4 to 1.1	This CSM study, Phase 2
(mg-N/L)	1.3	0.74	0.5 to 2.4	This CSM study, Phase 1
	0	0	0 to 2	Tyler et al., 1991
Total P	13.4	6.4	2.2 to 31.2	This CSM study, Phase 2
(mg-P/L)	7.7	2.0	5.7 to 11.1	This CSM study, Phase 1
	17.3	-	12 to 20	Crites & Tchobanoglous, 1998
	7.3	-	-	Harkin et al., 1979
Fecal coli.	-	-	1.4E+05 to 2.6E+07	This CSM study, Phase 2
(org/100mL)	-	-	4.0E+04 to 5.1E+06	This CSM study, Phase 1
	-	-	1.0E+06 to 1.0E+08	Crites & Tchobanoglous, 1998
	1.00E+06	-	-	Harkin et al., 1979

Table 4.3. Comparison of STE composition from Phase 2 with literature values.

"-" indicates information not available.

4.1.3 WSAS Soil Characteristics

A total of 3 soil cores (two within the infiltrative surface and one background location) were successfully collected and sampled at each of 4 WSAS (3 chamber and 1 gravel systems) studied during Phase 2. Three of the four systems exhibited some degree of effluent ponding while the fourth system was wet with ponding observable in surface depressions. Coring was expedited through the use of direct push core collection methods, larger diameter core barrels, and descriptive information gained during Phase 1 (e.g., location of subsurface system boundaries, system depth). Detailed results for each WSAS may be found in the Appendix (Table A.5) while a summary of the results follow.

Water content versus depth for the soil cores collected from all of the study sites in Phase 2 is shown in Fig. 4.1. Water content is generally highest near the infiltrative surface and declines with increasing depth below the infiltrative surface. Figure 4.2 illustrates the ammoniumnitrogen and nitrate-nitrogen data from soil core samples by depth interval for each site. Ammonium-nitrogen and nitrate-nitrogen appear to be present throughout the sampling depths at each site, with highest levels at depths closest to the infiltrative surface. At one location (home site Id# 16), ponding was present to the top of the chamber and a black anoxic layer ~3-in. (7.5 cm) thick was observed surrounding the outside of the chamber. Nitrification does not appear to be occurring at this location as indicated by the absence of nitrate in the soil cores (see Fig. 4.2). Similar to water content and ammonium-nitrogen and nitrate-nitrogen, available phosphorous and % organic matter decline with depth below the infiltrative surface (see graphs presented in Appendix A, Figs. A.5 and A.6).



Fig. 4.1. Water content in soil core samples by depth below the infiltrative surface. Background soil core data are excluded.

The results for fecal coliform bacteria levels with depth are summarized in Appendix A (Table A.5) while Fig. 4.3 provides a graphical summary of fecal coliform levels at soil coring depths for chamber versus gravel systems. As shown, the results for the chamber systems are comparable to those for the gravel systems. At all of the Phase 2 study sites, fecal coliform concentrations declined with depth and by 60 cm depth, fecal coliform bacteria were not detected (detection level of 1 cfu per g dry soil). It should be noted that fecal coliform bacteria was detected at 9 to 20 cfu/100mL in the background core at one location (home site Id# 14) collected outside of the infiltrative surface (Table A.5). The source of the fecal coliforms is unknown. The fecal coliform results observed in this Phase 2 study are similar to the Phase 1 results and previous results in the literature (Converse and Tyler 1998, Siegrist et al. 2000).



Fig. 4.2. Nitrate- and ammonium-nitrogen in soil samples by depth below the infiltrative surface (in mg/kg as N, dry soil). Note variation in y-axis scale for site Id# 16.



Fig. 4.3. Fecal coliform data in soil samples from four home sites (1 gravel and 3 chambers) sampled during Phase 2. Background soil core data are excluded.

4.2. Multicomponent Surrogate/Tracer Results

The concentrations of conservative tracer (bromide) and viral surrogates (MS-2 and PRD-1) measured in the STE at each home over time are shown in Fig. 4.4 with details provided in Appendix A (Tables A.6 through A.8). These results show a decline in the bromide concentration in the dosing tank that is consistent with the decline in concentration expected based on dilution due to incoming STE without bromide tracer. After five days, the bromide concentration had declined by 62 to 90% and had declined by 88 to 96% at the time of coring.



Fig. 4.4. Concentrations of the conservative tracer bromide and PRD-1 and MS-2 bacteriophages in the STE dosing tank over time at sites 10, 12, 14, and 16.

Initial levels of both MS-2 and PRD-1 were several orders of magnitude less than anticipated based on stock concentrations $(10^{15} \text{ and } 10^{13} \text{ pfu/mL} \text{ respectively})$ and tank size. The lower initial concentration might be attributed to potential adsorption of the bacteriophage onto solids present in the tank during mixing. Settling of the particles within the grab samples prior to analysis may have prevented detection/quantification. It should be noted that initial bromide concentration (1500 mg-Br/L) indicating tank volume estimates were acceptable. The levels of MS-2 and PRD-1 remained relatively unchanged through out the duration of the test (Fig. 4.4). These apparently unchanging virus levels may be due in part to analytical error and the potential for some growth of the bacteriophages in the STE. Results during the Phase 1 inactivation test indicated that some apparent growth of bacteriophage in the STE was possible at 4°C and 20°C (Fig 4.5) (Siegrist et al. 2000). More intensive monitoring of the temporal changes in surrogate/tracer concentrations was conducted during Phase 2 and appears to confirm a similar trend.



Fig. 4.5. Phase 1 inactivation of MS-2 and PRD-1 in STE during incubation at 20°C and 4°C. Note that all samples were run in duplicate and the average percent difference was 11%.

Analysis of the soil cores detected bromide in the extracted soil samples at up to 30 cm below the infiltrative surface confirming that STE from the tank had indeed been delivered to the location of the soil core and that sufficient time had elapsed for percolation through the soil to a depth of >30 cm (Fig. 4.6 and Appendix A, Table A.9). The absence of bromide in soil core extracts in the 55 to 60 cm interval is attributed to insufficient time between surrogate/tracer addition and soil coring for the added bromide to migrate into and through the depth interval of interest.

Soil core values for MS-2 and PRD-1 are also graphically depicted in Fig. 4.6 with detailed results presented in Appendix A, Table A.9. An overall trend of lower levels of surrogate virus with increasing depth below the infiltrative surface was observed. As with the conservative tracer, the absence of both surrogates in soil core extracts in the 55 to 60 cm interval could be attributed to insufficient time between surrogate/tracer addition and soil coring for the surrogate/tracer to migrate into and through the depth interval of interest. Fecal coliform bacteria also declined with depth below the infiltrative surface, but it should be noted that fecal coliform was detected in the background samples at home site Id# 14 (Appendix A, Table A.5).



Fig. 4.6. Bromide, MS-2 and PRD-1 levels in soil core extracts collected during Phase 2, 7 to 9 days following addition of surrogates and tracer to the STE being applied. Note: Detection limits are: bromide at 0.1 mg/L (ponding) or mg/kg dry soil; and MS-2 and PRD-1 at 1 pfu/mL (ponding) or g of dry soil. Zero values (blank bars) represent non-detects.

The relationship of MS-2 and PRD-1 to fecal coliform concentrations is of interest, since fecal coliforms are often used as indicators of microbial contamination. During Phase 1 a comparison of fecal coliforms to MS-2 and PRD-1 indicated that, under the conditions examined, fecal coliforms in soil extracts may be a reasonable indicator for the presence of virus at the same location. Results from Phase 2 confirm this relationship as all soil core extracts with virus also had fecal coliform concentrations (Appendix A, Table A.10).

The removal of bacteriophages was conservatively estimated based on the assumption that all bacteriophage detected in the extraction of soil solids were mobile in the soil pore water. Based on the average dry wt.% water content at each home site, the pfu/g dry soil values were converted to pfu/mL of pore water (Table A.11). These estimated pore water values were then compared to the dose concentration of MS-2 or PRD-1. The estimated pore water concentrations correspond to 98.2 to 99.9% removal efficiency for both MS-2 and PRD-1 within the top 15 cm below the infiltrative surface and 99.5 to 99.8% for MS-2 and 99.1 to 99.9% for PRD-1 at 30 cm below the infiltrative surface (Fig. 4.7, Table A.11). Because the conservative tracer was not detected at the 55-60 cm interval, treatment efficiencies were not estimated for this depth. Considering that the extracted bacteriophage may have occurred, it is reasonable to conclude that a 3-log removal of the applied viral surrogates was achieved at a total depth of 30 cm (maximum sample depth where surrogate/tracers were detected). Achievement of 3-log removal of virus after STE infiltration at 1 to 3 cm/d and percolation through 60 to 90 cm of natural soil is reasonable to achieve as shown in this and previous field studies (see Table 4.4).



Fig. 4.7. Estimated removal efficiency of MS-2 and PRD-1 bacteriophages with depth below the infiltrative surface at home site Id#s 10, 12, 14, and 16. Note: blank bars represent removal efficiencies lower than 95% (see Table A.11).

	Study	Virus and		
Investigator	characteristics/	concentrations	Method of	Findings
	location	applied	assessment	
This CSM study, Phase 2, 2001	1.73 cm/d HLR of STE to three mature chamber soil	Spiking of STE with MS-2 at \sim 1.0 to 4.3x10 ³ and	Soil core collection and extraction	99.9% removal after 30 cm
	absorption system and 0.84 cm/d HLR of STE to one mature gravel soil absorption system <i>Colorado</i>	PRD-1 at ~6.5 to 11.1x10 ³ pfu/mL		
This CSM study, Phase 1, 1999	2.7 cm/d HLR of STE to a mature chamber soil absorption system <i>Colorado</i>	Spiking of STE with MS-2 at 7.5x10 ⁴ and PRD- 1 at 1.5x10 ⁵ pfu/mL	Soil core collection and extraction	99.9% removal after 60 cm
Higgins et al., 1999	3cm/day HLR of STE to a buried sand filter constructed of medium sand <i>Massachusetts</i>	Indigenous MS-2 at $3x10^4$ pfu/mL in raw wastewater and $7.8x10^3$ in STE applied to sand	Pressure-free pan lysimeters placed during sand placement in buried lined cells	74.44% removal in septic tank 99.17% removal in 30cm ¹ 98.45% removal in 60 cm ¹ 99.79% removal in 152cm ¹
Oakley et al., 1999	Variable loading (0.81-6.5 cm/day) of STE to a soil absorption system in clay loam <i>California</i>	Indigenous $\phi X174$ at $1x10^{0}$ to $1x10^{4}$ pfu/mL in STE	Suction- lysimeters augured and driven into intact natural soil	1-log removal in recirculating gravel filter100% removal in 60 cm soil
Anderson et al., 1991	Onsite soil absorption systems and subdivisions on fine sandy soils <i>Florida</i>	Indigenous virus present in STE at 0.06 to 43.7 MPN of infectious units per L	Soil cores and extraction plus ground water samples	No Enterovirus detected in soil samples below the soil infiltration area at four homes At one home, virus was detected in shallow ground water (0.6 to 0.9 m) depth right under the system but not 3 m downgradient from it
Gilbert et al., 1976	Secondary effluent land applied at 100 m/year with cyclic flooding onto fine loamy sand <i>Arizona</i>	Indigenous Enterovirus at $1x10^3$ to $7x10^3$ pfu/100L in municipal effluent	Ground water sampling and analysis	99.99% removal in 3 to 9 m soil

Table 4.4. Results of field studies of virus treatment in wastewater soil absorption systems.

¹%removals shown in soil are based on the STE levels applied to the soil.

4.3. DISCUSSION

The interpretation of soil purification efficiency requires determination that wastewater did in fact reach the infiltrative surface location at which soil cores have been collected. Such an assessment can be made by integrated consideration of several factors, including: ponding (or

wetness), soil color, water content and nutrient levels in the soil core profile, as well as the presence of fecal coliform bacteria, the tracer and viral surrogates near the infiltrative surface. Based on these parameters, all soil cores were collected at locations where STE had infiltrated.

Ammonium-nitrogen and nitrate-nitrogen appear to be present throughout the sampling depths with the highest levels at depths closest to the infiltrative surface. These data are consistent with results found in laboratory lysimeters (Van Cuyk et al. 1999, Fischer 1999), the Phase 1 field study (Siegrist et al. 2000), and with recent published field research (Tyler and Converse 1998). Comparisons for nitrogen and fecal coliform demonstrate that the data collected in this study is generally consistent with previously reported results (Table 4.3). While a large degree of variation in constituent concentrations was observed between individual systems, and even among duplicate cores taken within the same system, the values measured for both system types were for all practical purposes, comparable.

During Phase 2, surrogate/tracer methods developed and lessons learned in Phase 1 were deployed at 4 additional sites enabling evaluation of five WSAS for virus treatment efficiency, including both aggregate-laden and aggregate-free systems. Bromide concentrations added in the STE dosing chamber were 1500 mg-Br/L to ensure continued high dosing of the bromide during the study. Frequent sampling and analysis of the STE (every 2 to 3 days) indicated that high levels of bromide were delivered to the infiltrative surface throughout the duration of this test. Bromide was detected in all soil cores from within the infiltrative surface with the exception of one site (home site Id#16). Frequent detection of bromide is attributed to the higher sustained concentrations of the conservative tracer delivered and the shorter time frame between surrogate/tracer addition and soil sampling. It is possible that bromide was not detected at home site Id#16 because the system was ponded to the top of the chamber (12 in. depth) and initial concentrations were attenuated to the extent that it's presence in the soil pore water was not detected. At home site Id#16, bromide was detected in the ponded STE as well as in the samples taken from infiltrative surface vents located at both ends of the infiltration field (12 to 183 mg-Br/L).

As noted during Phase 1 and confirmed in Phase 2, a relationship was observed between the concentrations of fecal coliforms and MS-2 and PRD-1 virus in soil samples. These data suggest that under the conditions examined, in soil extracts where fecal coliform bacteria are absent, then it is unlikely that virus exist at the same location.

Bacteriophage was detected in the surface cores (0-5 cm and 10-15 cm below the infiltrative surface) collected where ponding was observed. The results of the evaluation completed revealed that under the conditions examined, a 3-log treatment efficiency for virus was achieved at the 30-cm depth below the STE soil infiltrative surface. Removal of \geq 3 logs of virus during soil absorption of STE is consistent with the results of the CSM laboratory studies (Van Cuyk et al. 2001) as well as the results of previous field studies reported by other investigators (Table 4.4).

5.0 CONCLUSIONS

A field study was completed to monitor the performance of five mature wastewater soil absorption systems in Colorado to gain insight into the comparative performance of aggregate-free (chamber) and aggregate-laden (gravel) infiltration systems. Two phases of field activities have been conducted. Phase 1, initiated in 1999 was designed to characterize infiltration of domestic STE in more mature soil absorption systems under field conditions and included evaluation of virus treatment efficiency at one location. Phase 2 field testing, described in this report, was initiated in Fall 2000 as a companion to Phase 1 to expand the evaluation of virus treatment efficiency from one chamber WSAS to a total of 5 WSAS including both aggregate-laden and aggregate-free systems. During both phases, virus treatment efficiency was evaluated using a multicomponent mixture of MS-2 and PRD-1 bacteriophages and a conservative bromide tracer. Data collected at each site included residence characteristics, system design features, STE composition, occurrence and depth of ponding of the soil infiltrative surface, and bacteriophage, conservative tracer, fecal coliform bacteria and nutrient concentrations with depth below the infiltrative surface.

Based on the work completed during both Phase 1 and Phase 2 field evaluations, and with consideration of the related 3-D lysimeter research and previously reported findings (Van Cuyk et al. 1999, 2001; Siegrist et al. 1999, 2000; Siegrist and Van Cuyk 2001), the following conclusions have been drawn and several recommendations can be made.

- 1. The STE composition at the four individual study homes was typical of residential STE containing appreciable concentrations of pollutants. The average concentrations were: $cBOD_5 = 122 \text{ mg/L}$, TSS = 209 mg/L, total N = 58 mg-N/L, and total P = 13.4 mg-P/L. Fecal coliform bacteria ranged from 1.4 x 10⁵ to 2.64 x 10⁷ cfu/100mL.
- 2. The chamber systems varied in age from 3 to 10 yr, while the gravel system was 9 yr, old. The estimated HLR averaged 1.73 cm/d for the chamber systems, including both Phase 1 and Phase 2, compared to 0.84 cm/d for the gravel system. During Phase 2, three of the four systems (home site Id#s 10, 12, and 16) exhibited some degree of effluent ponding while the fourth system (home site Id#14) was wet with ponding observable in surface depressions. During soil coring (after surrogate/tracer addition) it was observed that one system (home site Id#16) was fully ponded. This system was not eliminated from consideration at this time due in part to schedule and reporting constraints (e.g., time required to identify an additional system, add surrogate/tracer to the STE, and allow sufficient time for migration of the surrogate/tracer into and through the infiltration area). It is also important to note that this system was designed as an infiltration/evapotransporation system relying on a fraction of the net loss of fluid from evapotransporation rather than percolation through the low permeable subsurface soils, and was sampled during the winter months when evapotransporation was at a minimum (i.e., dominant infiltration/negligible evapotransporation). Data from these four systems suggest comparable hydraulic performance with the chamber systems receiving a loading rate on average that was 106% higher (1.73 vs. 0.84 cm/d per unit of horizontal infiltrative surface area).

- 3. Monitoring of soil properties and pollutant concentrations with depth beneath the infiltrative surfaces revealed spatially variable concentrations. At most sites, pollutant concentrations declined with depth and by 60 cm depth, fecal coliform bacteria were not detected. The results of this effort also confirmed a relationship between fecal coliform concentrations measured in soil core samples to MS-2 and PRD-1 virus concentrations, observed during Phase 1.
- 4. The methodology for using a multicomponent mixture of viral surrogates (MS-2 and PRD-1) and a conservative tracer (bromide) to assess virus purification in a wastewater soil absorption system developed during Phase 1 was successfully applied under field conditions to four additional sites (3 aggregate-free systems and one aggregate-laden system). During Phase 1, \geq 3-log reductions in the applied MS-2 and PRD-1 viral surrogate concentrations were achieved at 60 cm below the infiltrative surface at one aggregate-free system. During Phase 2, 3-log reductions in the applied MS-2 and PRD-1 viral surrogate concentrations were achieved at 30 cm below the infiltrative surface (maximum sample depth where surrogate/tracers were detected).
- 5. Under the conditions examined in this study, the performance and virus treatment efficiency measurements made for the chamber systems were comparable to those determined for gravel systems, even though the chambers were estimated to be receiving a HLR of 0.42 gpd/ft² (1.73 cm/d) as compared to 0.21 gpd/ft² (0.84 cm/d) for the gravel system. The performance observations made under field conditions are consistent with the findings derived from 3-D lysimeter studies carried out under controlled laboratory conditions at CSM (Van Cuyk et al. 1999, 2001; Siegrist et al. 1999) and Phase 1 field studies (Siegrist et al. 2000; Siegrist and Van Cuyk 2001) and indicate that aggregate-free chamber systems and aggregate-laden gravel systems provided equivalent environmental protection.

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7.0 APPENDIX



Fig. A.1. Soil core locations at home site Id# 10, aggregate-laden.



Fig. A.2. Soil core locations at home site Id# 12, aggregate-free.



Fig. A.3. Soil core locations at home site Id# 14, aggregate-free.



Fig. A.4. Soil core locations at home site Id# 16, aggregate-free.



Fig. A.5. Available phosphorus in soil core samples by depth below the infiltrative surface (in mg-P/kg dry soil). Background soil core data are excluded.



Fig. A.6. Organic matter in soil core samples by depth below the infiltrative surface. Background soil core data are excluded.

Site	pН	Alk.	COD	cBOD ₅	TS	TSS	TN	NH ₄	NO ₃	ТР	FC
Id#		mg/L as CaCO ₃	mg/L	mg/L	mg/L	mg/L	mg/L as N	mg/L as N	mg/L as N	mg/L as P	cfu/100 mL
10	7.57	494	482	218	1035	162	52	49.9	0.6	11.9	1.40E+05
	-	-	481	-	1020	150	76	47.3	0.8	-	-
	7.49	508	576	230	1195	148	64	47.7	0.8	8.9	1.40E+05
	-	-	543	-	1155	245	60	48.2	0.5	13.9	-
	7.54	518	369	30.6	990	-	84	54.2	0.8	11.2	2.30E+05
	-	-	-	-	965	200	60	44.2	0.5	2.2	2.00E+05
	7.38	524	-	34.2	630	245	64	44.4	0.4	3.6	3.20E+05
	-	-	-	-	1080	275	72	44.0	0.4	15.0	2.80E+05
min.	7.38	494	369	30.6	630	148	52	44.0	0.4	2.2	1.40E+05
max.	7.57	524	576	230.0	1195	275	84	54.2	0.8	15.0	3.20E+05
median	7.52	513	482	126.1	1028	200	64	47.5	0.6	11.2	2.15E+05
ave.	7.50	511	490.2	128.2	1009	204	67	47.5	0.6	9.5	2.18E+05

 Table A.1.
 Characteristics of STE samples collected from Site Id# 10, gravel system.

- = no data.

Table A.2. Characteristics of STE samples collected from Site Id# 12, chamber system.

Site	pН	Alk.	COD	cBOD ₅	TS	TSS	TN	NH ₄	NO ₃	ТР	FC
Id#		mg/L as CaCO ₃	mg/L	mg/L	mg/L	mg/L	mg/L as N	mg/L as N	mg/L as N	mg/L as P	cfu/100 mL
12	8.32	476	376	-	1265	455	40	29.4	0.5	12	4.60E+06
	-	-	372	-	1255	550	48	28.8	0.5	12.2	-
	8.25	506	423	-	1300	140	72	31.4	0.6	13.2	8.20E+06
	-	-	403	-	1340	285	64	30.2	0.5	14.6	-
	8.55	580	442	42.6	1410	310	32	34.4	1	5.6	1.77E+07
	-	-	-	-	1435	375	32	37	ND	6.7	1.91E+07
	8.5	588	-	37.2	1415	365	32	32	0.8	4.8	2.64E+07
	-	-	-	-	1360	480	32	37.2	0.6	13.6	1.38E+07
min	8.25	476	372	37.2	1255	140	32	28.8	0.5	4.8	4.60E+06
max	8.55	588	442	42.6	1435	550	72	37.2	1.0	14.6	2.64E+07
median	8.41	543	403	39.9	1350	370	36	31.7	0.6	12.1	1.58E+07
ave.	8.41	537.5	403.2	39.9	1348	370	44	32.6	0.6	10.3	1.50E+07

- = no data.

ND = non detect

Site	pН	Alk.	COD	cBOD ₅	TS	TSS	TN	NH ₄	NO ₃	ТР	FC
Id#		mg/L as CaCO ₃	mg/L	mg/L	mg/L	mg/L	mg/L as N	mg/L as N	mg/L as N	mg/L as P	cfu/100 mL
14	7.34	540	688	-	1250	110	76	71	0.7	20.7	7.60E+06
	-	-	674	-	1190	140	80	68.6	0.6	19.5	-
	7.34	546	701	-	1235	135	76	75	0.5	19.6	7.80E+06
	-	-	703	-	1225	135	80	71	0.6	23.7	-
	6.98	546	786	389.4	1170	215	64	63.6	1.1	19.3	1.45E+07
	-	-	-	-	1200	190	68	66.4	1.1	9.7	4.30E+06
	7.23	578	-	67.2	1265	195	71	57.6	0.6	31.2	7.10E+06
	-	-	-	-	1255	210	68	59.4	0.7	16.8	2.40E+06
min	6.98	540	674	67.2	1170	110	64	57.6	0.5	9.7	2.40E+06
max	7.34	578	786	389.4	1265	215	80	75.0	1.1	31.2	1.45E+07
median	7.29	546	701	228.3	1230	165	74	67.5	0.7	19.6	7.35E+06
ave.	7.22	552.5	710.4	228.3	1224	166	73	66.6	0.7	20.1	7.28E+06

Table A.3. Characteristics of STE samples collected from Site Id# 14, chamber system.

- = no data.

Table A.4. Characteristics of STE samples collected from Site Id# 16, chamber system.

Site	pН	Alk.	COD	cBOD ₅	TS	TSS	TN	NH ₄	NO ₃	ТР	FC
Id#		mg/L as CaCO ₃	mg/L	mg/L	mg/L	mg/L	mg/L as N	mg/L as N	mg/L as N	mg/L as P	cfu/100 mL
16	7.12	620	657	-	1245	70	64	42.2	0.6	12.8	5.40E+06
	-	-	653	-	1250	80	64	42.2	0.6	14.6	-
	7.1	464	604	-	1145	35	40	43.8	0.7	13.2	-
	-	-	608	-	1215	105	40	41.2	0.6	10.8	-
	7.18	548	620	49.2	1045	75	40	41.6	0.9	5	5.20E+06
	-	-	-	-	1100	85	44	38.4	0.7	8.4	8.60E+06
	7.15	518	-	120	1225	145	52	39.2	0.6	17.9	6.00E+06
	-	-	-	-	1260	155	52	39.2	0.7	21.8	1.17E+07
min	7.10	464	604	49.2	1045	35	40	38.4	0.6	5.0	5.20E+06
max	7.18	620	657	120.0	1260	155	64	43.8	0.9	21.8	1.17E+07
median	7.14	533	620	84.6	1220	83	48	41.4	0.7	13.0	6.00E+06
ave.	7.14	537.5	628.4	84.6	1186	94	50	41.0	0.7	13.1	7.38E+06

- = no data.

Site	Sample	Ponding	Depth	Color	pН	WC	Org Mat		NH ₄	NO ₃	ТР	Avail P	FC
Id#	Location	(v/n)	(cm)			%	0/0	mg/kg as N	mg/kg as N	mg/kg as N	mo/ko as P	mo/ko as P	cfu/g dry soil
10	D2	(y/II)	0.5	10 VD 4/2		10.4	70	11	11	11	iiig/ kg us i	iiig/ kg us i	ND
10	D2	no, wet	10.15	$101 \text{ K}^{4/3}$	-	10.4	-	-	-	-	-	-	
			25.20	1.3 I K4/4	-	12.7	-	-	-	-	-	-	
			25-30	10YR4/4	-	11.8	-	-	-	-	-		
	D 4		55-60	10 Y K4/3	-	12.5	-	-	-	-	-	-	
	B4	yes	0-5	/.5YR4/3	/.1	11.3	0./	386.90	19.69	2.69	/21.5/	27	7.E+01
			10-15	10YR3/4	7.4	12.5	0.5	295.62	10.06	1.12	402.17	13	5.E+01
			25-30	10YR5/6	7.4	13.2	0.6	186.85	8.69	2.44	221.72	11	2.E+01
	2		55-60	7.5YR4/4	7.2	12.7	0.5	302.00	8.80	3.50	390.89	10	ND
12	Bkgd ²	NA	0-5	10YR5/2	-	20.6	-	-	-	-	-	-	ND
			10-15	10YR6/3	-	10.2	-	-	-	-	-	-	ND
			25-30	10YR6/3	-	14.3	-	-	-	-	-	-	ND
			55-60	10YR6/4	-	1.9	-	-	-	-	-	-	ND
	B1	no, wet	0-5	-	8.6	23.7	1.0	836.61	9.16	7.82	807.65	27	7.E+03
			0-5	-	-	25.3	-	-	-	-	-	-	2.E+04
			10-15	10YR5/3	8.9	24.1	0.6	402.85	8.94	4.85	558.38	7	4.E+02
			10-15	10YR5/3	-	24.2	-	-	-	-	-	-	7.E+02
			25-30	10YR5/3	9.2	26.0	0.6	382.67	5.73	7.24	569.91	2	4.E+01
			25-30	10YR5/3	-	26.0	-	-	-	-	-	-	4.E+01
			55-60	10YR5/3	9.2	18.1	0.5	305.88	5.98	5.68	447.98	2	ND
			55-60	10YR5/3	-	18.1	-	-	-	-	-	-	ND
	B2	yes	0-5	10YR4/4	8.5	26.9	0.5	513.33	10.94	9.83	629.93	7	3.E+02
			10-15	10YR5/6	8.9	27.6	0.4	298.60	6.31	6.62	395.31	2	ND
		Ì	25-30	10YR4/4	8.9	20.1	0.5	337.83	6.22	4.19	575.52	2	2.E+02
			55-60	10YR4/4	8.6	26.6	0.4	327.10	9.69	10.44	380.04	2	ND

Table A.5.Summary of soil core data collected in Phase 2.1

- = no data, NA = not applicable, ND = not detected.
 ¹ Results expressed on dry weight basis.
 ² Bkgd = background cores, soil samples taken at stated distance below infiltrative surface.

Site	Sample	Ponding	Depth	Color	pН	WC	Org Mat	TN	NH ₄	NO ₃	ТР	Avail P	FC
								mg/kg as	mg/kg as	mg/kg as			
Id#	Location	(y/n)	(cm)			%	%	Ν	N	N	mg/kg as P	mg/kg as P	cfu/g dry soil
14	Bkgd	NA	0-5	10YR5/6	-	15.0	-	-	-	-	-	-	9.E+00
			10-15	10YR5/6	-	12.9	-	-	-	-	-	-	3.E+01
			25-30	10YR5/6	-	13.0	-	-	-	-	-	-	9.E+00
			55-60	10YR5/6	-	13.7	-	-	-	-	-	-	2.E+01
	B1	no, wet	0-5	10YR4/4	8.6	30.5	1.0	744.04	8.96	13.31	704.43	1	2.E+02
			10-15	10YR4/4	8.8	24.3	0.9	604.70	5.70	12.90	487.40	17	ND
			25-30	10YR4/4	8.8	23.7	0.8	496.39	6.32	12.52	446.77	1	ND
			55-60	10YR5/4	9.0	26.8	0.7	401.79	5.69	11.77	436.65	1	ND
	B2	yes	0-5	mottled	8.1	35.6	1.3	1110.91	46.95	3.36	971.93	21	9.E+02
			0-5	mottled	-	34.4	-	-	-	-	-	-	1.E+03
			10-15	10YR4/4	8.8	24.7	0.8	607.24	7.60	2.14	583.54	1	2.E+02
			10-15	10YR4/4	-	24.7	-	-	-	-	-	-	7.E+01
			25-30	10YR4/4	8.9	24.1	0.7	447.41	6.30	3.27	572.41	1	2.E+01
			25-30	10YR4/4	-	23.8	-	-	-	-	-	-	1.E+01
			55-60	10YR4/4	8.8	22.8	0.7	413.33	6.35	3.62	441.95	2	ND
			55-60	10YR4/4	-	22.7	-	-	-	-	-	-	ND
16	Bkgd	NA	0-5	7.5YR3/3	-	22.4	-	-	-	-	-	-	ND
	_		10-15	7.5YR3/3	-	22.4	-	-	-	-	-	-	ND
			25-30	10YR5/6	-	31.1	-	-	-	-	-	-	ND
			55-60	10YR5/6	-	25.0	-	-	-	-	-	-	ND
	B1	yes	0-5	10YR4/6	8.0	24.2	0.6	594.55	117.30	1.16	394.18	3	ND
			0-5	10YR4/6	-	24.4	-	-	-	-	-	-	ND
			10-15	10YR4/6	7.9	24.6	0.8	575.79	54.61	1.11	421.53	4	ND
			10-15	10YR4/6	-	23.1	-	-	-	-	-	-	ND
			25-30	10YR4/6	8.5	21.8	0.6	410.46	11.90	1.22	466.15	3	ND
			25-30	10YR4/6	-	-	-	-	-	-	-	-	ND
			55-60	10YR5/4	8.2	28.5	0.6	387.36	19.55	1.83	292.51	3	ND
			55-60	10YR5/4	-	27.7	-	-	-	-	-	-	ND
	B2	yes	0-5	mottled	8.1	26.3	0.8	722.71	113.43	0.15	319.22	4	1.E+01
			10-15	10YR4/3	8.0	26.0	0.8	568.89	22.15	1.86	311.61	6	ND

Summary of soil core data collected in Phase 2.¹ Table A.5 (cont).

- = no data, NA = not applicable, ND = not detected. ¹Results expressed on dry weight basis.

 2 Bkgd = background cores, soil samples taken at stated distance below infiltrative surface.

Elapsed	S	ite Id# 1	0	S	ite Id# 1	2	Site Id# 14			Site Id# 16		
Time	Ave	SD	CV	Ave	SD	CV	Ave	SD	CV	Ave	SD	CV
(days)	(mg/L)	(mg/L)		(mg/L)	(mg/L)		(mg/L)	(mg/L)		(mg/L)	(mg/L)	
0	1827	4	0.002	1870	12	0.006	1401	550	0.39	1226	206	0.17
2	-	-	-	181	11	0.06	309	115	0.37	1077	773	0.72
3	358	4	0.01	-	-	-	-	-	-	-	-	-
5	237	16	0.07	-	-	-	-	-	-	-	-	-
6	-	-	-	189	22	0.12	282	22	0.08	464	51	0.11
7	151	7	0.05	158	3	0.02	168	8	0.04	-	-	-
9	-	-	-	-	-	-	-	-	-	44	15	0.34

Bromide concentrations in the STE dosing tank with time. Table A.6.

Table A.7.MS-2 concentrations in the STE dosing tank with time.

Elapsed	Elapsed Site Id# 10			Site Id# 12			Site Id# 14			Site Id# 16		
Time	Ave	SD	CV	Ave	SD	CV	Ave	SD	CV	Ave	SD	CV
(days)	(pfu/mL)	(pfu/mL)		(pfu/mL)	(pfu/mL)		(pfu/mL)	(pfu/mL)		(pfu/mL)	(pfu/mL)	
0	4333	1041	0.24	873	406	0.46	1420	674	0.47	1933	1793	0.93
2	-	-	-	375	66	0.18	617	161	0.26	1550	779	0.50
3	407	169	0.42	-	-	-	-	-	-	-	-	-
5	780	893	1.14	-	-	-	-	-	-	-	-	-
6	-	-	-	800	346	0.43	1067	231	0.22	621	149	0.24
7	1313	595	0.45	533	306	0.57	633	46	0.07	-	-	-
9	-	-	-	-	-	-	-	-	-	340	295	0.87

Table A.8.	PRD-1 concentrations in the STE dosing tank with time
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Table A	.0. Г	$\overline{\text{KD-1}}$	meenti			E uosin	ig talik v		5.			
Elapsed	Si	ite Id# 1	0	Si	ite Id# 1	2	Site Id# 14			Site Id# 16		
Time	Ave	SD	CV	Ave	SD	CV	Ave	SD	CV	Ave	SD	CV
(days)	(pfu/mL)	(pfu/mL)		(pfu/mL)	(pfu/mL)		(pfu/mL)	(pfu/mL)		(pfu/mL)	(pfu/mL)	
0	11167	2566	0.23	9500	2291	0.24	430	99	0.23	6500	1000	0.15
2	-	-	-	217	161	0.74	617	161	0.26	317	144	0.46
3	329	235	0.72	-	-	-	-	-	-	-	-	-
5	367	153	0.42	-	-	-	-	-	-	-	-	-
6	-	-	-	933	924	0.99	400	200	0.50	147	31	0.21
7	227	46	0.20	741	1090	1.47	333	115	0.35	-	-	-
9	-	-	-	-	-	-	-	-	-	1030	1372	1.33

				Br	MS-2	PRD-1
Site	Sample	Ponding	Depth	mg/kg	pfu/g dry soil	pfu/g dry soil
Id#	Location	(y/n)	(cm)	dry soil	or pfu/mL	or pfu/mL
10	B2	no, wet	0-5	1.6	ND	ND
			10-15	0.8	ND	ND
			25-30	0.5	ND	ND
			55-60	0.2	ND	ND
	B4	yes	ponding	143.5	940.0	196.0
			0-5	18.0	126.0	ND
			10-15	23.7	18.0	ND
			25-30	12.4	ND	ND
			55-60	0.2	ND	ND
12	Bkgd	NA	0-5	< 0.1	ND	ND
	_		10-15	< 0.1	ND	ND
			25-30	< 0.1	ND	ND
			55-60	< 0.1	ND	ND
	B1	no, wet	0-5	22.4	210.0	ND
			0-5 D	24.9	214.0	ND
			10-15	20.5	21.0	ND
			10-15 D	21.3	ND	ND
			25-30	9.5	ND	ND
			25-30 D	8.0	ND	ND
			55-60	0.1	ND	ND
			55-60 D	0.1	ND	ND
	B2	yes	ponding	42.7	640.0	64.0
			0-5	19.1	22.0	ND
			10-15	7.9	ND	ND
			25-30	3.6	ND	ND
			55-60	< 0.1	ND	ND

Table A.9.Bromide and MS-2 and PRD-1 bacteriophages in soil with depth.

B = corehole location

Bkgd = background corehole location

D = duplicate

NA = not applicable

ND = non detect

- = not analyzed (insufficient sample volume)

				Br	MS-2	PRD-1
Site	Sample	Ponding	Depth	mg/kg	pfu/g dry soil	pfu/g dry soil
Id#	Location	(y/n)	(cm)	dry soil	or pfu/mL	or pfu/mL
14	Bkgd	NA	0-5	0.1	ND	ND
			10-15	0.4	ND	ND
			25-30	0.3	ND	ND
			55-60	<0.1	ND	ND
	B1	no, wet	0-5	7.8	ND	ND
			10-15	0.1	ND	ND
			25-30	< 0.1	ND	ND
			55-60	<0.1	ND	ND
	B2	yes	ponding	174.3	800.0	200.0
			0-5	21.7	497.0	ND
			0-5 D	24.7	244.0	ND
			10-15	2.9	ND	ND
			10-15 D	3.3	ND	ND
			25-30	0.9	ND	ND
			25-30 D	0.6	ND	ND
			55-60	<0.1	ND	ND
			55-60 D	<0.1	ND	ND
16	Bkgd	NA	0-5	1.8	ND	ND
			10-15	-	ND	ND
			25-30	-	ND	ND
			55-60	-	ND	ND
	B1	yes	ponding	61	52.0	2000.0
			0-5	0.6	ND	ND
			0-5 D	0.4	ND	ND
			10-15	0.6	ND	ND
			25-30	0.5	ND	ND
			25-30 D	0.7	ND	ND
			55-60	1.0	ND	ND
			55-60 D	0.9	ND	ND
	B2	yes	ponding	62.0	ND	200.0
			0-5	0.3	ND	ND
			10-15	0.2	ND	ND

Table A.9. (cont).Bromide and MS-2 and PRD-1 bacteriophages in soil with depth.

B = corehole location

Bkgd = background corehole location

D = duplicate

NA = not applicable

ND = non detect

- = not analyzed (insufficient sample volume)

Depth	FC	MS-2	PRD-1	Ra	tios
(cm)	cfu/g dry soil	pfu/g dry soil	pfu/g dry soil	FC/MS-2	FC/PRD-1
5	2 E+04	214	ND	115.03	
5	7 E+03	210	ND	34 94	
5	1 E+03	244	ND	5 50	
5	9.E+02	497	ND	1.75	
5	3.E+02	22	ND	14.91	
5	2.E+02	ND	ND		
5	7.E+01	126	ND	0.57	
5	1.E+01	ND	ND		
5	ND	ND	ND		
5	ND	ND	ND		
5	ND	ND	ND		
15	7.E+02	ND	ND		
15	4.E+02	21	ND	20.07	
15	2.E+02	ND	ND		
15	7.E+01	ND	ND		
15	5.E+01	18	ND	3.05	
15	ND	ND	ND		
15	ND	ND	ND		
15	ND	ND	ND		
15	ND	ND	ND		
15	ND	ND	ND		
15	ND	ND	ND		
30	2.E+02	ND	ND		
30	4.E+01	ND	ND		
30	4.E+01	ND	ND		
30	2.E+01	ND	ND		
30	2.E+01	ND	ND		
30	1.E+01	ND	ND		
30	ND	ND	ND		
30	ND	ND	ND		
30	ND	ND	ND		
30	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		
60	ND	ND	ND		

Table A.10. Relationship of fecal coliforms and bacteriophage in soil core samples collected below the infiltrative surface (Background locations removed from comparison).

ND = non detect

					MS	-2						
Home Site Id#	10			12			14			16		
Depth below IS	5cm	15cm	30cm	5cm	15cm	30cm	5cm	15cm	30cm	5cm	15cm	30cm
	126	18	1	210	21	1	1	1	1	1	1	1
	1	1	1	214	1	1	497	1	1	1	1	1
				22	1	1	244	1	1	1	1	1
Median (pfu/g)	63.5	9.5	1	210	1	1	244	1	1	1	1	1
Median (pfu/mL)	522.63	78.19	8.23	879.77	4.19	4.19	921.10	3.78	3.78	3.97	3.97	3.97
Initial Tank Conc.	4333	4333	4333	873	873	873	1420	1420	1420	1933	1933	1933
% Reduction	87.94	98.20	99.81	-0.77	99.52	99.52	35.13	99.73	99.73	99.79	99.79	99.79

 Table A.11. Estimated virus treatment efficiency.

PRD-1

Home Site Id#	10			12			14			16		
Depth below IS	5cm	15cm	30cm									
	1	1	1	1	1	1	1	1	1	1	1	1
	1	1	1	1	1	1	1	1	1	1	1	1
	1	1	1	1	1	1	1	1	1	1	1	1
Median (pfu/g)	1	1	1	1	1	1	1	1	1	1	1	1
Median (pfu/mL)	8.23	8.23	8.23	4.19	4.19	4.19	3.78	3.78	3.78	3.97	3.97	3.97
Initial Tank Conc.	11167	11167	11167	9500	9500	9500	430	430	430	6500	6500	6500
% Reduction	99.93	99.93	99.93	99.96	99.96	99.96	99.12	99.12	99.12	99.94	99.94	99.94

Note: Bacteriophage detection limits of 1 pfu/g dry soil.